

SPLICE GIRDERS IN THE AIR.
SET G3 TO G1 IN ORDER, WITH DIAPHRAGMS ATTACHED TO GIRDERS AS THEY ARE PICKED.

NOTES:

GIRDERS A, C, AND E MAY BE LIFTED WITH EITHER 15-TON OR 25-TON BEAM

GIRDERS B AND D WILL BE LIFTED WITH 25-TON BEAM CLAMPS.
GIRDER C REQUIRES USE OF A MIN. 16' LONG, 16-TON SPREADER BEAM AND

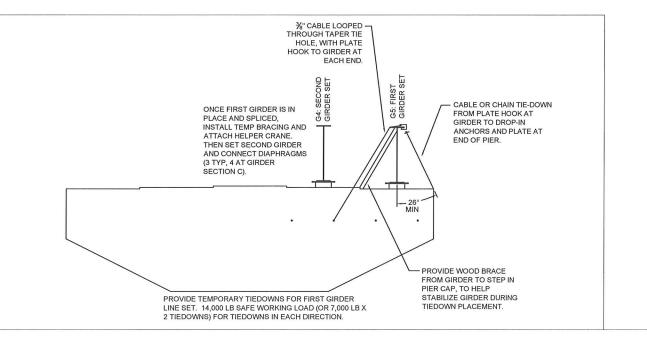
THE OTHER GIRDERS MAY BE PICKED FROM A SINGLE POINT.

CONNECT BETWEEN GIRDER LINES WITH 3 DIAPHRAGMS PER GIRDER SECTION TYP, AND 4 DIAPHRAGMS ON GIRDER SECTIONS C.
CONNECT SPLICE AND MAKE DIAPHRAGM CONNECTIONS BEFORE RELEASING

CRANES FROM A GIRDER LINE.

SET STEEL WHEN WINDSPEEDS WILL NOT EXCEED 30 MPH, OR PER CRANE OPERATING REQUIREMENTS. LOWER WINDSPEEDS MAY BE NEEDED FOR SAFE HANDLING OF STEEL.

			GIRDER	ADD FOR	ADD FOR	WEIGHT TO	138HSL	RTC80110	ATC3210
	GIRDERS	LEN GTH	WEIGHT	RIGGING	DIAPH/SPLICE	SET	MAX. RAD.	MAX. RAD.	MAX. RAD.
(Α	82	20,000	2500+750	885+1300	25,435			70
/	В	89	31,200	2500+750	885+1300	36,635	38		
	С	108	26,150	2500+1200	1,180	31,030	43		
	D	89	31,300	2500+750	885+2600	38,035		45	
	E	70	17,100	2,500	885	20,485		70	75





SHEET 1 OF 1 46A STEEL ERECTION PLAN WATERBURY IM 089-2 (43) JULY 7, 2016 BECK & BELLUCCI, INC. FRANKLIN, NEW HAMPSHIRE

CALCULATIONS FOR:

Waterbury, VT IM 089-2(43) 506.55, Structural Steel, Plate Girder

Steel Handling and Erection Plan

July 7, 2016

Prepared by:

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Design References:

NHDOT Standard Specifications, Section 550 (for girder stability) LRFD Steel Design, 2nd Ed., by William T. Segui LRFD Design Manual, by the AISC

Plans and Specifications:

VTrans Contract Drawings ARC Steel Shop Drawings

Description of Work:

The bridge is multiple spans. Provide temporary lateral support for the first girder line placed while placing the second girder line. Set the first two girder sections across all girder lines before proceeding to the next two girder sections. The permanent diaphragms are used during erection, to provide lateral support for subsequent girders. After all girders are erected, the bolts will be torqued and inspected.

Equipment Used:

Link-Belt 138 HSL Link-Belt RTC 80110 Link-Belt ATC 3210

Link-Belt RTC 8050 (helper crane for temporary girder support)

Check girder stability:

From NHDOT spec:

- 3.14.2.4 Lifting. Proper consideration shall be given to guard against lateral buckling when lifting beams and girders. Generally speaking, straight beams and girders 30 in. (760 mm) in depth or deeper, lifted according to the following criteria should be stable. Dimensions for length/width ratios are in feet and b equals the minimum width of the flange in compression.
- (a) One Crane (Overhang using a single line pickup at the girder centerline with or without a spreader) For the unsupported overhang length, d, the maximum d/b ratio should not exceed 35 nor should the overhang length exceed 50 ft.
- (b) Two Cranes (Distance between beam clamps at the girder ends on a two-point pick-up) For the unsupported length, a, between beam clamps on a two-point pick-up, the a/b ratio should not exceed 85 nor should the distance between pick-up points exceed 100 feet.

The girders must be kept stable while lifting, so the NHDOT standard specification 550.3.14.2.4, for lifting of steel girders, will be used to check the girder pick points.

Max interior support: a/b < 85; $a_{max} = 85*1.33' = 113'$, use 100'. **100' max interior support spacing.**

Girder A:

Flange width b = 16" = 1.33' (note: use 25-ton beam clamps for 16"-24" wide flange.)

Max overhang: d/b < 35; $d_{max} = 35*1.33' = 46'$. Girder length is 82' / 2 = 41' overhang if picked in the middle. 41' < 46', **ok for 1 pick point**.

Girder B:

Flange width b = 20" = 1.66' (note: use 25-ton beam clamps for 16"-24" wide flange.)

Max overhang: d/b < 35; $d_{max} = 35*1.66' = 58'$, use 50' max. Girder length is 89' / 2 = 45' overhang if picked in the middle. 45' < 50', **ok for 1 pick point**.

Girder C:

Flange width b = 16" = 1.33' (note: use 25-ton beam clamps for 16"-24" wide flange.)

Max overhang: d/b < 35; $d_{max} = 35*1.33' = 46'$. Girder length is 108' / 2 = 54' overhang if picked in the middle. N.G. – need spreader beam.

108' - 46'*2 = 16', need 16' min spreader beam with 31 k = 16 ton capacity.

Girder D:

Flange width b = 20" = 1.66' (note: use 25-ton beam clamps for 16"-24" wide flange.)

Max overhang: d/b < 35; $d_{max} = 35*1.66' = 58'$, use 50' max. Girder length is 89' / 2 = 45' overhang if picked in the middle. 45' < 50', **ok for 1 pick point**.

Girder E:

Flange width b = 20" = 1.66' (note: use 25-ton beam clamps for 16"-24" wide flange.)

Max overhang: d/b < 35; $d_{max} = 35*1.66' = 58'$, use 50' max. Girder length is 70' / 2 = 35' overhang if picked in the middle. 45' < 50', **ok for 1 pick point**.

A note about pick overhangs: I understand it is common construction practice to use the above support distances in combination with additional loading from diaphragms, overhang brackets, etc. Such extra weight is relatively small; for this girder, the self-weight is at least 244 lb/ft, vs. approx. 20 lb/ft for diaphragms and hangers.

Load to Pick:

The weight of the pick is based on a single girder, plus any diaphragms and splice plates, and an allowance for rigging (sufficient to include typical spreader beams and beam clamps).

The steel has the following max. piece weights, from the structural steel drawings: Diaphragms: 295 lb. ea. Allow for 3 diaphragms on typ girders, 4 on girder C Splice Plates (max): 125*2 + 65*4 + 150*2 + 30*2 = 870 lb * 1.5 for bolts = 1300 lb at end of A&B girders, and both ends of D girders.

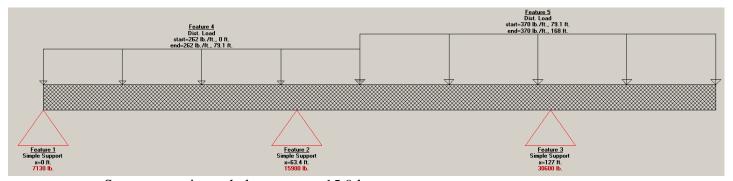
Rigging: allow for 2500 lb for block and cable. Girder C has additional 1200 lb for a 20' speader beam

Girder stability with temporary crane supports:

50-ton crane is to hold the first girders while the other cranes set the second girder line.

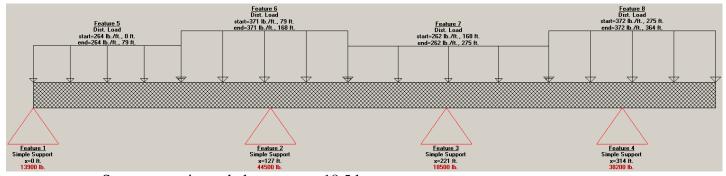
Load on 50-ton crane:

Girders A-B: crane is to lift at midpoint of span 1.



Support reaction at helper crane = 15.9 kRTC8050 has capacity = 17.8 k at R = 40° , ok radius per drawing.

Girders A-D:



Support reaction at helper crane = 18.5 k

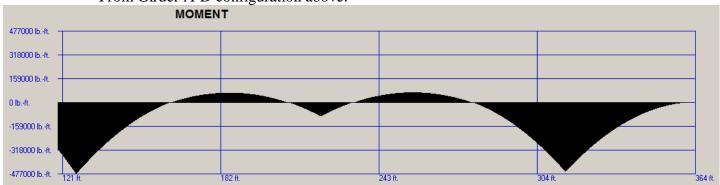
RTC8050 has capacity = 37.9 k at R = 25', ok radius per drawing.

Girder E:

Less unbraced length than girdera A-B. ok by inspection.

Check stability of middle span, with helper crane picking girder. Unbraced length is from last diaphragm on girder section B to pier 2.

From Girder A-D configuration above:



Lb = 153' = 1836 in from last diaphragm on girder B to pier 2 Unbraced length is comparatively very long vs Lb and Lr; assume elastic LTB. Use section properties for girder C.

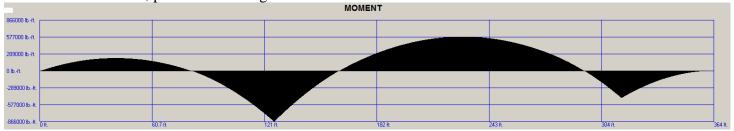
$$\begin{array}{lll} Mn = Cb^*pi()/Lb^* \; sqrt\{\; E^*Iy^*G^*J + (pi()^*E/Lb)^*2 \; * \; Iy^*Cw \; \} < Mp \\ Cb & 2.99 \\ Lb & 1836 \; in \\ E & 29000 \; ksi \\ Iy & 598 \; in^*4 \\ G & 11200 \; ksi \\ J & 9.18 \; in^*4 \\ Cw & 793000 \; in^*6 \\ Mp = Fy \; * \; Zx = 50 \; ksi \; * \; 1749 \; in^*4 = 87450 \; k\text{-}in \\ \\ Mn = & 8788.448 \; k\text{-}in \\ < Mp, \; use \; value \; shown. \\ \\ u = & 477 \; k\text{-}ft \\ 5724 \; k\text{-}in \\ \end{array}$$

Mu = 5724 k-in 0.65 of Mn, ok.

also, the helper crane will provide some amount of lateral support

Temporary Bracing at Supports for First Girder:

Temporary bracing must resist 2% or the bending moment resisted by half the girder section, plus wind loading.



The max moment while using the helper crane (assume helper crane is absent to maximize moments) is found to be at pier 1 & 2 while girders A-D are connected. Mmax = 866 k-ft = 10,400 k-in.

For a T-section of half the girder, find the centroid (use smallest section):

	Α)	ybar	A*ybar
1/2 web		20.25	18	364.50
flange		14	36.4375	510.13
sums:		34.25		874.63
Ybar =		25.54 i	nches fron	n centroid

Compressive load from bending = 10400 k-in / 2 for compressive component / 25.54 in Ybar = 204 k

Brace load = 204 k * 0.02 = 4 k horizontal brace load.

Wind pressure: take as 30 mph. Wind may add to the brace load, say for 40 mph, pressure (psf) = 0.00256*30 mph² = 2.3 psf * 6' height * 2.0 for drag on a flat plate = 28 lb/ft / 2 for top flange * 157' max tributary length = 2.2 k horizontal wind force.

Total horizontal brace load = 4 k + 2.2 k = 6.2 k. Total axial brace load for 45-deg brace = $6.2 \text{ k} / \sin(26) = 14 \text{ k}$ axial load total. Divide among several braces.

For **1 brace each side of girder**, each brace must support 14 k axial load. By inspection, this force is resisted by min 2 runs of 3/8" cable (SWL = 14,840 lb / 2 = 7.4 k ea) or chains connected to plate hooks at the girder, and tied down to adjacent anchor bolts. Note the bracing material is to have SWL = 14,000 lb.

Worker Protection:

Workers will be allowed on the erected steel only while all girders are braced with diaphragms or temporary bracing as indicated above, or the workers are in manlifts or man baskets. To allow worker access to the steel, fall protection posts and cable will be installed on girders prior to erection. The DBI/Sala Protecta fall protection system requires cable supports every 60' at maximum, so three posts will be used on the 124' girders, and two posts on the 38' girders. After splicing, the fall protection lines will be strung the whole length of the bridge.

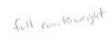
Attachments:

Crane Charts
Beam Clamps Information
Fall Protection Posts Information
Steel Erection Plan, sheet 1/1



138 HSL 10' of Boom Tube Boom Load Chart

Load	Boom Length – ft (m)												
Hadius ft (m)	40 (12.2)	50 (15.2)	60 (18.3)	70 (21.3)	80 (24.4)	90 (27.4)	100 (30.5)	110 (33,5)	120 (36.6)	130 (39.6)			
11 (3.4)	160,0 (72.6)												
12 (3.7)	160.0 (72.6)	160.0 (72.6)											
13 (4.0)	151.8 (68.9)	151.6 (68.8)											
14 (4.3)	141.6 (64.2)	141.4 (64.1)	141.1 (64.0)										
15 (4.6)	132.6 (60.1)	132.4 (60.1)	132.2 (60.0)		- 2								
16 (4.9)	124.7 (56.6)	124.5 (56.5)	124.3 (56.4)	123.9 (56.2)									
17 (5.2)	117.6 (53.3)	117.5 (53.3)	117.3 (53.2)	117.0 (53.1)									
18 (5.5)	111.1 (50.4)	111.2 (50.4)	111.0 (50.3)	110.7 (50.2)	109.0 (49.4)								
19 (5.8)	101.7 (46.1)	101.9 (46.2)	102.0 (46.3)	102.1 (46.3)	102.1 (46.3)	99.3 (45.0)							
20 (6.1)	93.7 (42.5)	93.9 (42.6)	94.0 (42.6)	94.1 (42.7)	94.1 (42.7)	94.0 (42.6)							
25 (7.6)	67.0 (30.4)	67.1 (30.4)	67.2 (30.5)	67.2 (30.5)	67.1 (30.4)	67.1- (30.4)	67.0 (30.4)	66.9 (30.3)	66.8 (30.3)				
30 (9.1)	51.8 (23.5)	51.9 (23.5)	52.0 (23.6)	52.0 (23.6)	51.9 (23.5)	51.8 (23.5)	51.7 (23.5)	51.6 (23.4)	51.5 (23.4)	51.4 (23.3)			
35 (10.7)	42.0 (19.1)	42.2 (19.1)	42.2 (19.1)	42.1 (19.1)	42.1 (19.1)	42.0 (19.1)	41.9 (19.0)	41.8 (19.0)	41.6 (18.9)	41.5 (18.8)			
40 (12.2)	35.1 (15.9)	35,3 (16.0)	35.3 (16.0)	35.3 (16.0)	35.2 (16.0)	35.1 (15.9)	35.0 (15.9)	34.9. (15.8)	34.7 (15.7)	34.6 (15.7)			
50 (15.2)		26.3 (11.9)	26.4 (12.0)	26.4 (12.0)	26.3 (11.9)	26.2 (11.9)	26.0 (11.8)	25.9 (11.7)	25.8 (11.7)	25.6 (11.6)			
60 (18,3)			1	20,8 (9,4)	20.7 (9.4)	20.6 (9.3)	20.4 (9.3)	20.3 (9.2)	20.2 (9.2)	20,0 (9,1)			
70 (21.3)					16.9 (7.7)	16.7 (7.6)	16.6 (7.5)	16.5 (7.5)	16.3 (7.4)	16.2 (7.3)			
80 (24.4)						14.0 (6.4)	13.8 (6.3)	13.7 (6.2)	13.5 (6,1)	13.4 (6.1)			
90 (27.4)							11.7 (5.3)	11.6 (5.3)	11.4 (5.2)	11.3 (5.1)			
100 (30.5)								9.9 (4.5)	9.8 (4.4)	9.6 (4.4)			
110 (33.5)									8.4 (3.8)	8.3 (3.8)			
120 (36.6)										7.1 (3.2)			



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Main Boom Lift Capacity Charts - Imperial

		450000		(A	CONCERNATION CONTRACTOR	MARKET WASH		Pounds)		4000			100
Radius			W 15		1 17.0	oom Ler							Radius
(ft)	40	50	60	70	80	90	100	110	120	130	140	150	(ft)
10	*220,000	107,200	105,400	101,500									10
12	187,600	107,200	105,400	101,500	101,300								12
15	164,600	107,200	105,400	101,500	96,600	81,900	56,500						15
20	123,400	107,200	105,400	101,500	80,800	73,200	65,900	56,000					20
25	95,800	95,400	94,000	87,800	69,000	62,700	60,100	56,000	58,000				25
30	77,200	76,900	76,600	76,300	59,900	55,000	53,500	56,000	51,300	48,600	38,400	30,000	30
35		63,700	63,400	63,200	53,200	53,000	53,500	51,800	45,400	43,100	38,400	30,000	35
40		52,400	52,000	52,000	53,200	52,800	52,800	46,300	40,600	38,500	37,000	30,000	40
45			43,600	44,200	44,700	42,900	43,000	41,700	36,500	34,600	33,300	30,000	45
50			36,100	36,900	37,300	37,600	35,600	35,700	33,000	31,300	30,200	29,100	50
55				31,300	31,800	32,100	30,200	30,300	30,000	28,500	27,500	26,500	55
60				26,800	27,300	27,600	27,900	25,900	25,900	25,900	25,100	24,200	60
65				7.	23,700	24,000	24,300	23,600	22,400	22,400	22,400	22,100	65
70					20,700	21,100	21,300	21,500	19,900	19,500	19,500	19,500	70
75					300000000000000000000000000000000000000	18,600	18,800	19,000	18,500	17,100	17,200	17,200	75
80						16,500	16,800	17,000	17,200	15,100	15,100	15,200	80
85							14,900	15,200	15,400	13,300	13,300	13,400	85
90							13,300	13,600	13,800	11,700	11,800	11,800	90
95								12,200	12,400	10,400	10,400	10,400	95
100								10,900	11,100	9,100	9,120	9,200	100
105									10,000	8,100	8,100	8,100	105
110								- 1	9,000	7,100	7,100	7,100	110
115										6,200	6,200	6,300	115
120										5,400	5,400	5,500	120
125											4,700	4,800	125
130		10									4,000	4,100	130
135												3,500	135
140					1000							2,900	140

Link-Belt ATC 3210: 5654-1013-53 Prefiminary

FULL WEIGHTS

Raidus	Boam Length (ft)											
(1)	44.3	58.9-59.8	73.5-75.9	88.6-92.4	103.7-107.5	119.3-124.1	134.9-136.8	151.9-153.3	167-168.9	183.6-186.5	200.1	Radii (ft)
8	420,000 **		1									8
10	300,000	297,300	262,200	212,200	120000							10
12	268,200	269,500	256,800	212,200								12
15	233,200	235,200	232,200	205,500	128,900					0.55		15
20	189,400	192,100	191,000	184,600	172,300	129,700	91,400					20
25	157,800	160,900	160,000	158,700	153,000	120,900	91,400	73,000	41,200			25
30	133,000	137,200	136,300	135,000	136,500	118,300	85,500	73,000	55,000	41,300		30
35	111,300	115,700	117,000	118,700	115,300	107,800	79,100	73,000	55,000	41,300	33,000	35
40		99,400	100,700	100,500	99,200	98,900	73,500	69,700	65,000	41,300	33,000	40
45	7	87,400	87,800	87,700	86,500	88,000	88,800	63,800	55,000	41,300	33,000	45
50		76,100	77,500	77,400	77,400	77,900	64,100	58,600	53,500	41,300	33,000	50
55	2010000	ACCUPATIONS.	70,700	70,800	70,800	69,400	80,400	54,100	50,500	41,300	33,000	55
60	1.		63,700	64,200	63,800	62,400	57,000	50,100	47,600	40.200	33,000	60
65			55.900	58,200	57,700	56,300	54,000	46,500	44,300	37,890	33,000	65
70				53,000	52,500	51,300	51,300	43,300	41,400	35,600	33,000	70
75	2000	1 2 2 2 7		48,500	48,000	46,800	48,100	40,500	28,700	33,700	32,600	75
80				42,200	44,100	42,900	44,100	37,900	36,400	31,900	30,800	80
85				100000000000000000000000000000000000000	40,100	40,900	40,100	35,000	34,200	30,200	29,100	85
90					36,600	38,000	36,600	33,400	32,200	28,600	27,600	90
95	A sheeped	4-10-201			33,600	34,900	33,700	31,700	30,300	27,200	26,200	95
100						32,300	31,000	30,400	28,600	25,800	25,000	100
105	BALL TO		100	1		29,900	28,500	29,100	27,100	24,400	23,800	100
110						25,000	26,400	27,100	25,000	23,000	22,400	110
115	100000	-		100	1000000	20,600	24,400	25,100	24,100	21,600	21,400	110
120							23,300	23,300	22,300	20,400	20,400	120
125	CARLES OF	500	100	300000	150000		21,000	21,700	20,700	19,400	19,600	126
130								20,200	19,200	18,300	18,700	130
135	STATE OF THE PERSON NAMED IN				10000	1 1 1 1 1 1 1		18,800	17,800	17,100	17,700	136
140								17,600	16,700	15,900	16,700	140
145	de de la constantina della con		Jan Salar			desired to	A PRITTING		15,500	15,000	15,600	146
150									14,500	14,400	14,500	150
155	24 125	10 10 100	100000				2010 775	1 1000	13,500	13,800	13,500	156
160									10,200	13,200	12,500	160
165	7 27 17	Committee of the		2000		CONTRACTOR	155 777	7 1 77		12,500	11,700	166
170										11,700	10,800	170
175	200000	THE RESIDENCE	2757507	THE RESERVE		1 22 1 2 2 1		9347-111		9,300	10,100	175
180											9,400	180
185		and the same of	Tours of the last	10000			description.			100000	8,700	165

Link-belt RTC 8050 II:

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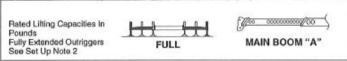
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Min.Bm.

Ang.Cap.

0 (44.5)

10,100



Load		35.5 Ft.			40 Ft.	
Radius (Ft.)	x°	360°	Over Front	×°	360°	Over Front
10	68.0	100,000	100,000	70.5	78,400	78,400
12	64.5	73,900	75,400	67.5	73,100	73,100
15	58.5	63,200	64,400	62.5	63,000	63,800
20	48.0	50,300	51,300	54.0	50,100	51,200
25	34.5	39,000	40,900	44.0	38,900	40,700
30				31.0	30,800	32,300
Min.Bm. Ang.Cap.	(30.0)	17,800	17,800	0 (34.5)	15,300	15,300
Load		50 Ft.			60.3 Ft.	
Radius (Ft.)	×°	360°	Over Front	×°	360°	Over Front
10	75.0	72,600	72,600			
12	72.5	65,600	65,600	76.5	50,900	50,900
15	69.0	57,500	57,500	73.5	46,900	46,900
20	62.5	47,600	47,600	68.5	39,200	39,200
25	55.5	38,500	40,200	63.0	33,400	33,400
30	48.0	30,500	32,100	57.5	28,700	28,700
35	39.0	24,800	26,100	51.0	24,600	25,200
40	27.5	19,100	20,400	44.0	18,900	20,200

10,100

36.0

26.0

0 (54.8) 14,900

11,800

6,500

Rated Lifting Capacities In Pounds	H H	Form 100 100 100
Fully Extended Outriggers See Set Up Note 2	FULL	MAIN BOOM "B"

Load		35.5 Ft.			40 Ft.		_	50 Ft.	
Radius (Ft.)	x°	360°	Over Front	×°	360°	Over Front	×°	360°	Over Front
10	68.0	100,000	100,000	70.5	37,900	37,900	74.5	37,900	37,900
12	64.5	73,900	75,400	67.5	37,900	37,900	72.5	37,900	37,900
15	58.5	63,200	64,400	62.5	37,900	37,900	69.0	37,900	37,900
20	48.0	50,300	51,300	54.0	37,900	37,900	62.5	37,900	37,900
25	34.5	39,000	40,900	44.0	37,900	37,900	55.5	37,900	37,900
30				31.0	31,300	32,900	48.0	31,900	33,500
35							39.0	26,100	27,500
40					-		27.5	20,800	22,100
Min.Bm Ang Cap	(30.0)	17,800	17,800	0 (34.5)	14,700	14,700	0 (44.5)	9,900	9,900
Load		60 Ft.			70 Ft.			80 Ft.	
Radius	~ 0	15/10/2%	Over	~ 0	0.000.000	Over	J. 0	360°	Over
(Ft.)	X	360°	Front	X	360°	Front	X.	360	Front
10	77.5	37,900	37,900		100 to 100 to		1		
12	76.0	37,900	37,900	78.0*	37,900	37,900			TAYARIA Barana
15	73.0	37,900	37,900	76.0	37,900	37,900	78.0*	35,400	35,400
20	68.0	37,900	37,900	72.0	37,900	37,900	74.5	34,700	34,700
25	62.5	37,900	37,900	67.5	37,900	37,900	71.0	34,200	34,200
30	56.5	32,300	33,900	62.5	32,500	32,800	67.0	30,300	30,300
35	50.5	26,500	27,800	57.5	26,700	28,100	63.0	26,900	27,200
40	43.5	21,200	22,500	52.5	21,400	22,700	58.5	21,500	22,800
45	35.5	17,100	18,200	46.5	17,300	18,400	54.0	17,400	18,500
50	25.0	13,900	14,900	40.5	14,200	15,200	49.0	14,300	15,300
55	THE STATE OF		1.18822	33.0	11,900	12,700	44.0	12,100	12,800
60			-	23.5	10,000	10,700	38.0	10,200	10,900
65				20.0	10,000	1.0,1.00	31.0	8,600	9,300
70							22.0	7,300	7,900
MinBm Ang Cap	0 (54.5)	7,000	7,000	0 (64.5)	5,000	5,000	0 (74.5)	3,500	3,500
		90 Ft.	Service Control		100 Ft.	10 10 10	101000000	110 Ft.	
Load Radius (Ft.)	×°	360°	Over Front	×°	360°	Over	×°	360°	Over
20	77.0	28,900	28,900		m Settle	PERM		20,800,000	- TOTA
25	74.0	28,200	28,200	76.0	24,000	24,000	77.5	19,500	19,500
30	70.5	24,800	24,800	73.0	22,500	22,500	75.0	19,500	19,500
35	67.0	22,000	22,000	70.0	19,900	19,900	72.5	18,300	18,300
40	63.5	19,700	19,700	67.0	17,800	17,800	70.0	16,400	16,400
45	59.5	17,500		63.5	15,900	15,900	67.0	14,600	14,600
		1000000	17,800	60.5	14,400	14,400	64.0	100000000000000000000000000000000000000	10000000
50	55.5	14,400	15,400	1000		SCHOOL ST	75.7	13,200	13,200
55	51.0	12,200	12,900	56.5	12,200	13,000	61.0	12,100	12,100
60	46.5	10,300	11,000	53.0	10,300	11,100	57.5	10,400	11,000
65	41.5	8,700	9,400	49.0	8,800	9,500	54.0	8,900	9,600
70	36.0	7,500	8,100	44.5	7,500	8,200	50.5	7,600	8,200
75	29.5	6,400	6,900	40.0	6,500	7,100	47.0	6,500 .	7,100
80	21.0	5,400	6,000	34.5	5,500	6,100	42.5	5,600	6,200
85				28.5	4,700	5,200	38.5	4,800	5,300
90				20.5	4,000	4,500	33.5	4,100	4,600
95					/		27.5	3,500	3,900
100				1			20.0	2,900	3,400
MinBm Ang Cap	0 (84.5)	2,400	2,400	0 (94.5)	1,600	1,600	0 (104.5)	900	900

16,000

12,800

6,500

BEAM CLAMPS

Beam clamps provide an efficient method for handling wide flange beam sections and plate girders. When lifting, they grip the beam at three points. When properly balanced and safely guided, the beam can be handled even if the clamp is slightly off center lengthwise.

Good safety procedures providing control of the lifted beam must be used. Beams should be gripped as near the center as possible. Snubbing lines at each end must be used to control excessive twisting or swinging, and to guide the beam to its proper place. Each lifting situation may have specific safety demands which should be used as required.

Beam clamps eliminate the need for slings, chokers, and spreader bars. The weight of the beam clamp automatically opens its tongs, which slide under the flanges of the beam. When the clamp is lifting, its center plate and gripping tongs work against each other—the heavier the beam, the greater the clamping pressure.

Model "NS" clamps have a recessed base to accept studs welded to a beam surface.

FlangeGrip

3 244 23

3 234

484 30

Width Range Min. Max.

4 to 10

7 to 17

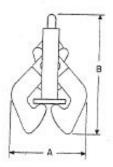
7 to 17

16 to 24

16 to 24

16 to 38

16 to 36



Working

Load Limit

5

15

15

25

25

35

Tons

Model

No.

F-5

F-15

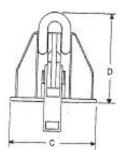
F-25

F-35.

NS-35

NS-25

NS-15



48

48

64

64

23

22 1/4

22 1.4

27 1/2

27 1/2

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48

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53

58

58

37 1/4

37 1/4

53

16

16

16

16

24

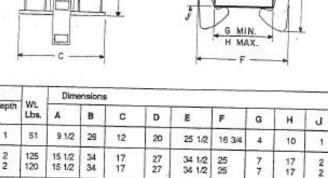
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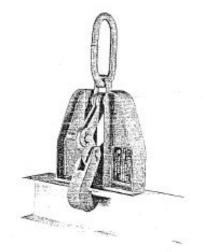
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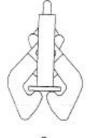
3

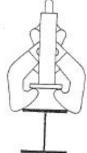
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4

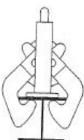








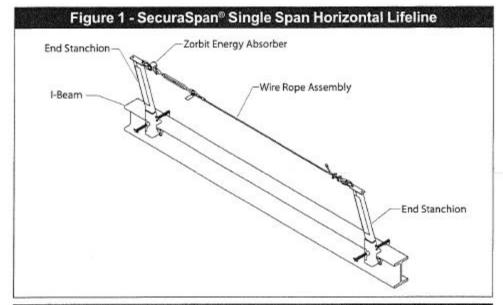


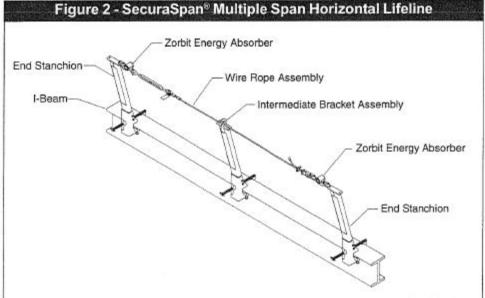


0 1	-L	or raises			L-M
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44					
+ 11				1	- 1
Lil		N.	BASE	- 1	j
-			a nase		

Model Number	NS B Dimer	ase Isions, I	nches				
	S	С	N	T	M	L	P
NS-15 NS-25 NS-35	5 1/2 6	16 1/2 22 1/4 27 1/2	6 1/2	2 1/2 4 4 1/2	6 1/2 7 3/4 9	1/2 3/4 3/4	3/4 3/4 3/4

WARNING: Decreasing the load by bumping or substantial imbalance can, under certain circumstances, loasen the grip. Do not use on flange widths less than those specified on the name plate. Do not exceed working load limit.





1.0 APPLICATION

- 1.1 PURPOSE: SecuraSpan® Horizontal Lifeline systems (HLL) are designed to be used as an anchoring means for up to six personal fall arrest systems (PFAS). The SecuraSpan system may be used in many situations where a combination of horizontal mobility and fall protection is needed. Figures 1 and 2 show two configurations of the SecuraSpan HLL systems.
- 1.2 LIMITATIONS: The following limits apply to the installation and use of the SecuraSpan Horizontal Lifeline System. Other limitations may apply.

IMPORTANT: OSHA regulations state that horizontal lifelines shall be installed and used under the supervision of a qualified person (see below for definition) as part of a complete personal fall arrest system that maintains a safety factor of at least two.

Qualified Person: An individual with a recognized degree or professional certificate, and extensive knowledge and experience in the subject field, who is capable of design, analysis, evaluation, and specification in the subject work, project, or product. Refer to OSHA 1910.66, 1926.32, and 1926.502.

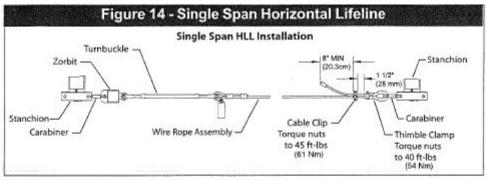
- A. HORIZONTAL LIFELINE SPAN: The maximum horizontal lifeline span length is 60 ft. (18.3 m). The system length can be extended by using multiple spans. See Figure 2. The span length must be reduced when clearance is limited. See section 3.0 for clearance information
- B. ANCHORAGES: SecuraSpan® Horizontal Lifeline (HLL) systems must only be installed to anchorages capable of meeting the strength requirements specified in Section 2.4.
- C. SYSTEM CAPACITY: The capacity of single span systems is two persons. The capacity of multiple span systems is two persons secured on each span with a maximum of six people installed on the system. The maximum weight of each person, including tools and clothing, is 310 lbs (141 kg).
- D. CONNECTING SUBSYSTEM: Each person's connecting subsystem (energy absorbing lanyard or SRL) must limit fall arrest forces to 900 lbs. (4 kN) or less. See section 2.1.
- E. FREE FALL: Rig and use the personal fall arrest system such that the maximum potential free fall does not exceed government regulatory and subsystem manufacturer's requirements. See section 3.0 and subsystem manufacturer's instructions for more information.
- F. SWING FALLS: See Figure 3.

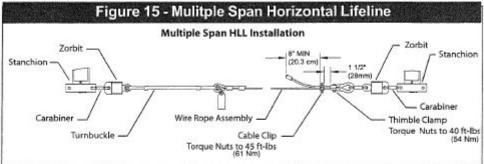
 Swing falls occur when the anchorage point is not in line vertically with the worker. The force of striking an object in a swing fall may cause serious injury or death. Minimize swing falls by working as directly in line with the anchorage point as possible. Do not permit a swing fall if injury could occur. Swing falls will significantly

Figure 3 - Swing Fall Hazard

Point

Connecting Subsystem





is 6 in. (15.25 cm) or less, with no weight on the wire rope. The turnbuckle will not over tension the wire rope.

Step 6. After pre-loading the system, re-torque all cable clips to values specified previously.

NOTE: Two independent SecuraSpan HLL systems may be terminated at the same stanchion.

3.3 OPERATION:

A. PERSONAL FALL ARREST SYSTEM COMPONENTS: Inspect and don the full body harness according to manufacturer's instructions. Attach the connecting subsystem (energy absorbing lanyard or SRL) to the dorsal connection on the harness.

WARNING: Risk of swing falls is greater when using an SRL. Swing falls significantly increase the clearance required to arrest a fall and may result in serious injury or death. To avoid swing fall hazards, do not work beyond the end stanchions or at excessive distances to either side of the HLL system. Do not climb above the HLL system.

B. CONNECTING TO THE HLL SYSTEM: Approach the work area using the appropriate access equipment. Connect the personal fall arrest system (the free snap hook on the enegry absorbing lanyard or carabiner attached to the SRL) to the horizontal lifeline.